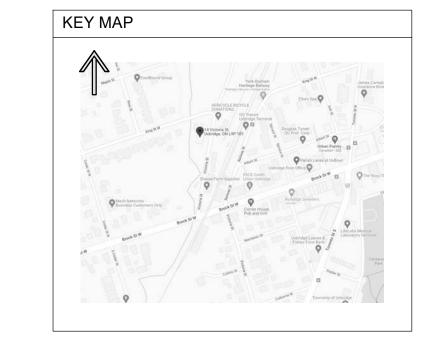


ONTARIO BUILDING CODE DATA MATRIX, PART 11 - BASIC RENOVATION OF EXISTING BUILDING					BUILDING CODE REFERENCE	
11.1	EXISTING BUILDING CLASSIFICATION	DESCRIBE EXISTING USE: CONSTRUCTION INDEX: HAZARD INDEX: [X] NOT APPLICABLE (NO CHANG	GE OF MAJOR (OCCUPANCY)	11.2.1.	
11.2	ALTERATION TO EXISTING BUILDING IS:	BASIC RENOVATION EXTENSIVE RENOVATION	[X]		11.3.3.1.	
11.3	REDUCTION IN PERFORMANCE LEVEL:	STRUCTURAL BY INCREASE IN OCCUPANT LOAD BY CHANGE OF MAJOR OCCUPANO PLUMBING SEWAGE SYSTEM		[]YES []YES []YES []YES []YES	11.4.2.1. 11.4.2.2 11.4.2.3 11.4.2.4 11.4.2.5	
11.4	COMPENSATING CONSTRUCTION	STRUCTURAL INCREASE IN OCCUPANT LOAD CHANGE IN MAJOR OCCUPANCY PLUMBING SEWAGE SYSTEM	ON[X] ON[X] ON[X] ON[X] ON[X]	[]YES (EXPLAIN)	11.4.3.2 11.4.3.3 11.4.3.4 11.4.3.5 11.4.3.6	
11.5	COMPLIANCE ALTERNATIVE PROPOSED	[X] NO [] YES (GIVE NUMBERS)			11.5.1	

EXISTING	PROPOSED
EXISTING	PROPOSED
400.0 m ²	446.4 m ²
99.1 m²	99.1 m ²
111.1 m ²	111.1 m ²
610.2 m ²	656.6 m ²
	99.1 m ²

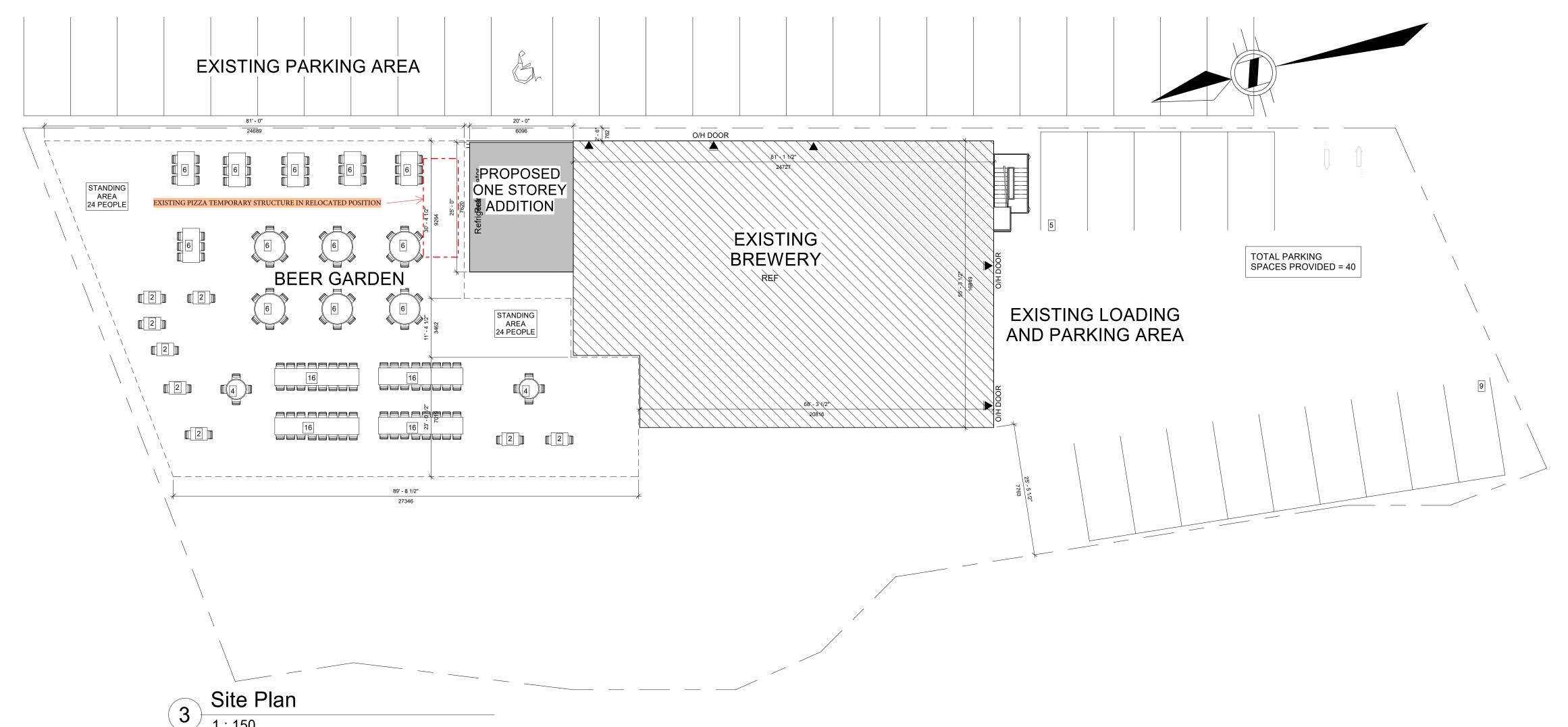


No.	Description	Date
1	ISSUED FOR REVIEW	MAY 23/24

WASHROOMS PROVIDED			
OCCUPANT LOAD IN BUILDING = 140 PEOPLE			
3 WATER CLOSETS REQUIRED FOR EACH SEX			
3 WATER CLOSETS FOR WOMEN			
2 WASTER CLOSETS + 1 URINAL FOR MEN			
5 - 1 STAFF WASHROOM PROVIDED			

EXIT WIDTHS		
3RD FLOOR OCCUPANT LOAD:	5	WIDTH REQUIRED: 8mm PER PERSON 40mm TOTAL REQUIRED
		WIDTH PROVIDED: 836mm
2ND FLOOR OCCUPANT LOAD:	40	WIDTH REQUIRED: 8mm PER PERSON 320mm TOTAL REQUIRED WIDTH PROVIDED: 1069mm
GROUNDFLOOR OCCUPANT LOAD:	140	WIDTH REQUIRED: 6.1mm PER PERSON 854mm TOTAL REQUIRED WIDTH PROVIDED: 2032mm

VICTORIA STREET





2 Black Court Aurora, Ontario, L3G 8A1 647-219-7651 st.theodor@bell.net



PROPOSED ADDITION

14 VICTORIA STREET UXBRIDGE, ON

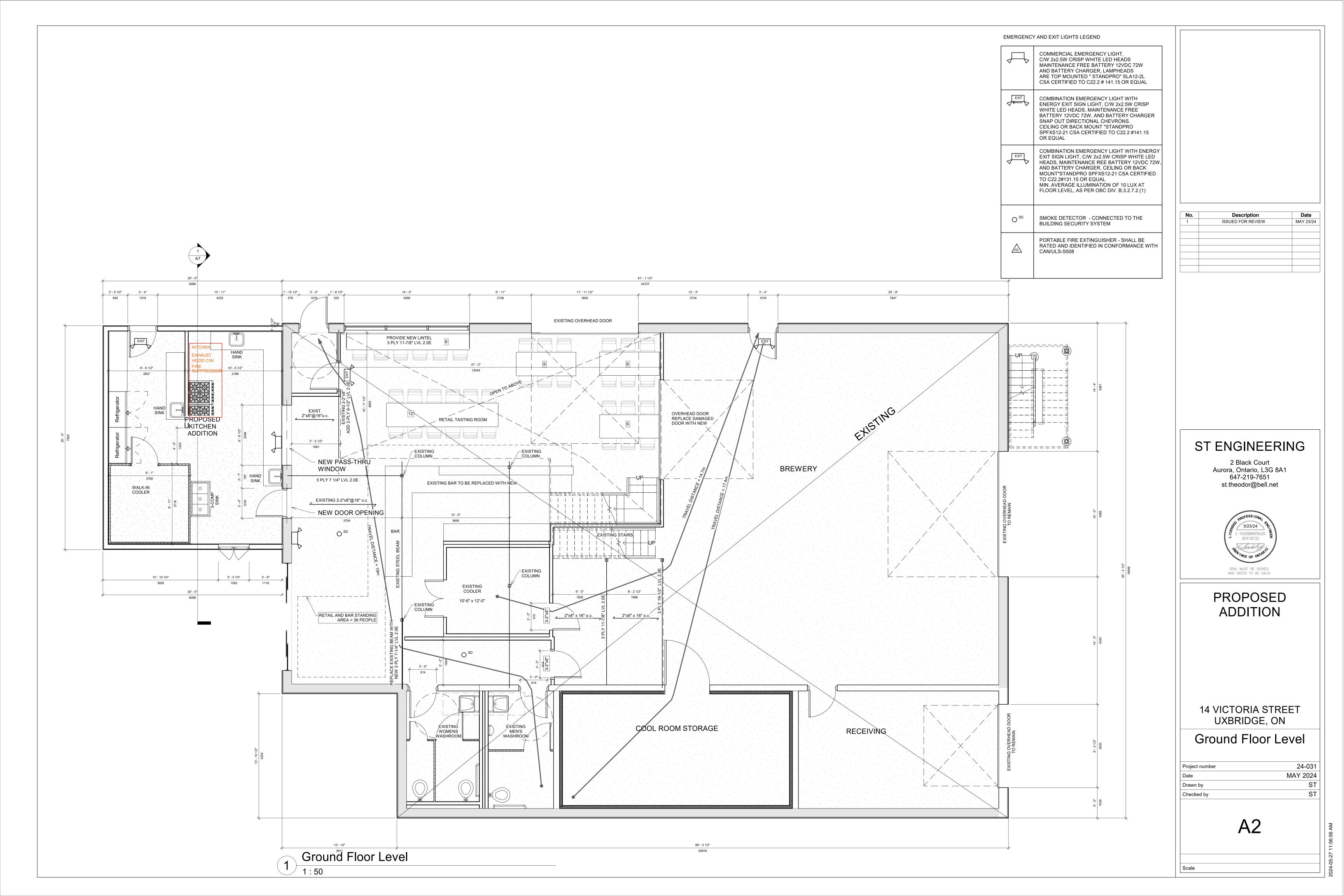
Site Plan

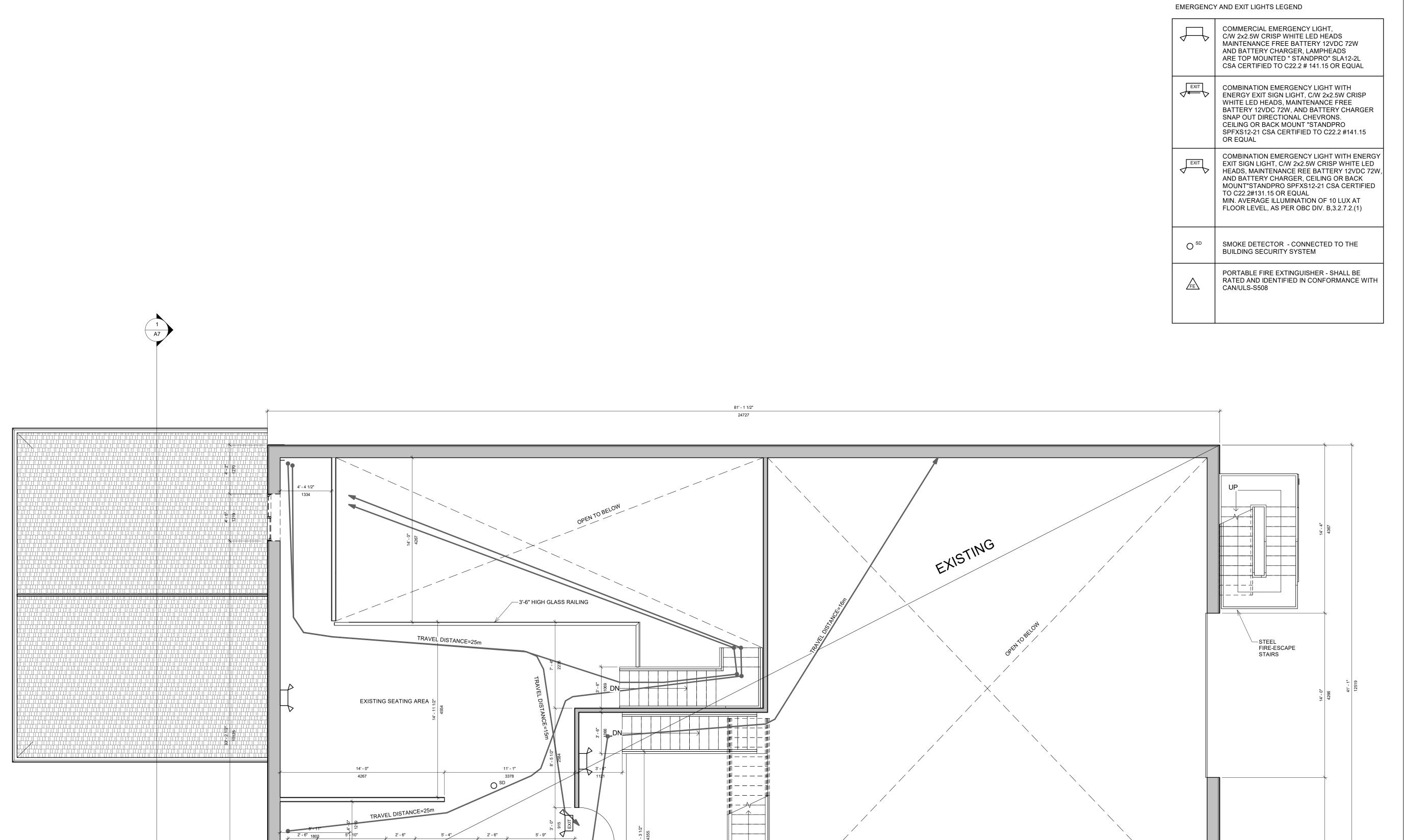
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Date	MAY 2024
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12' - 0 1/2"

WOMENS WASHROOM

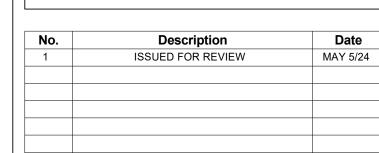
Existing 2nd Floor Level

LIVE LOAD = 4.8 kPa
 SUPERIMPOSED DEAD LOAD = 0.25 kPa

STORAGE ROOM

WASHROOM_

/--3'-6" HIGH GUARD RAIL



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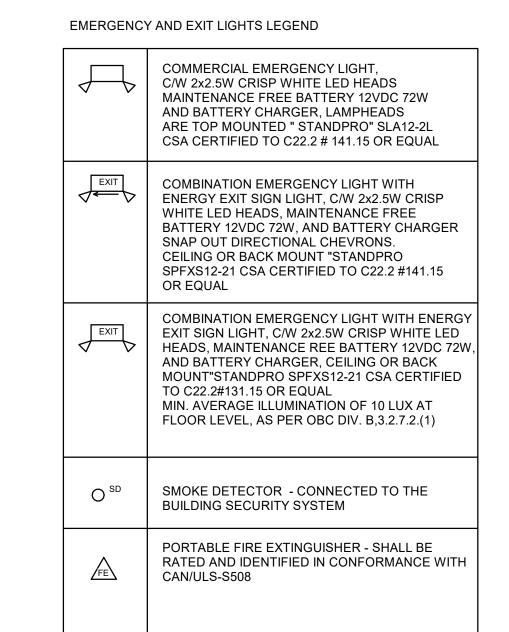
2nd Floor Level

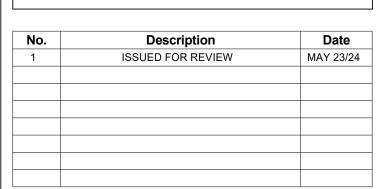
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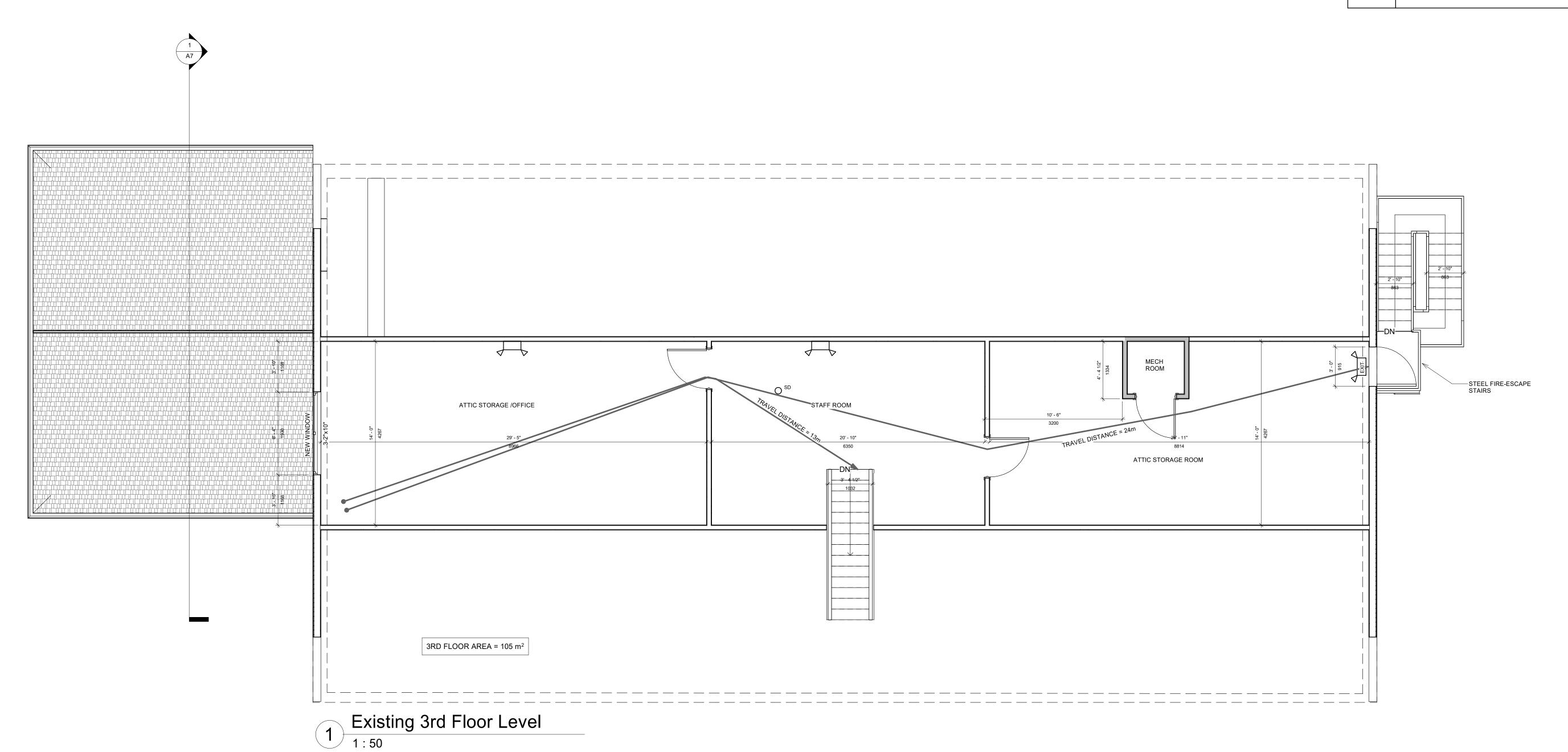
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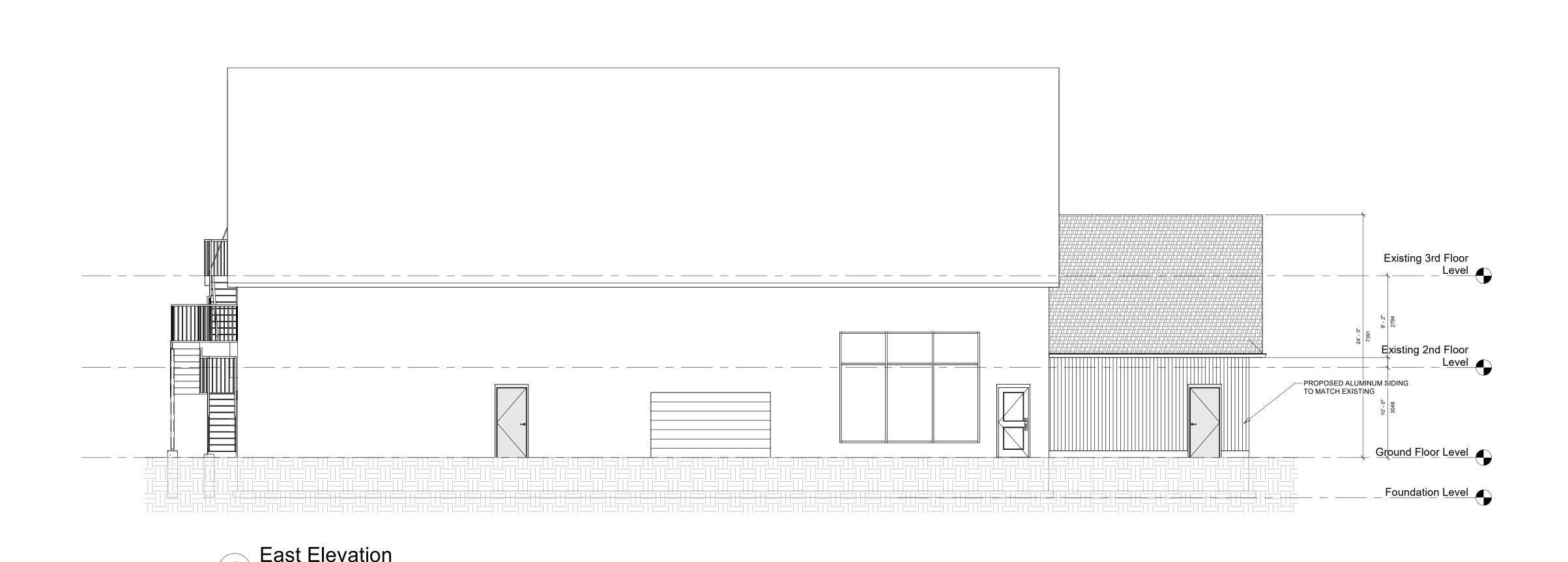
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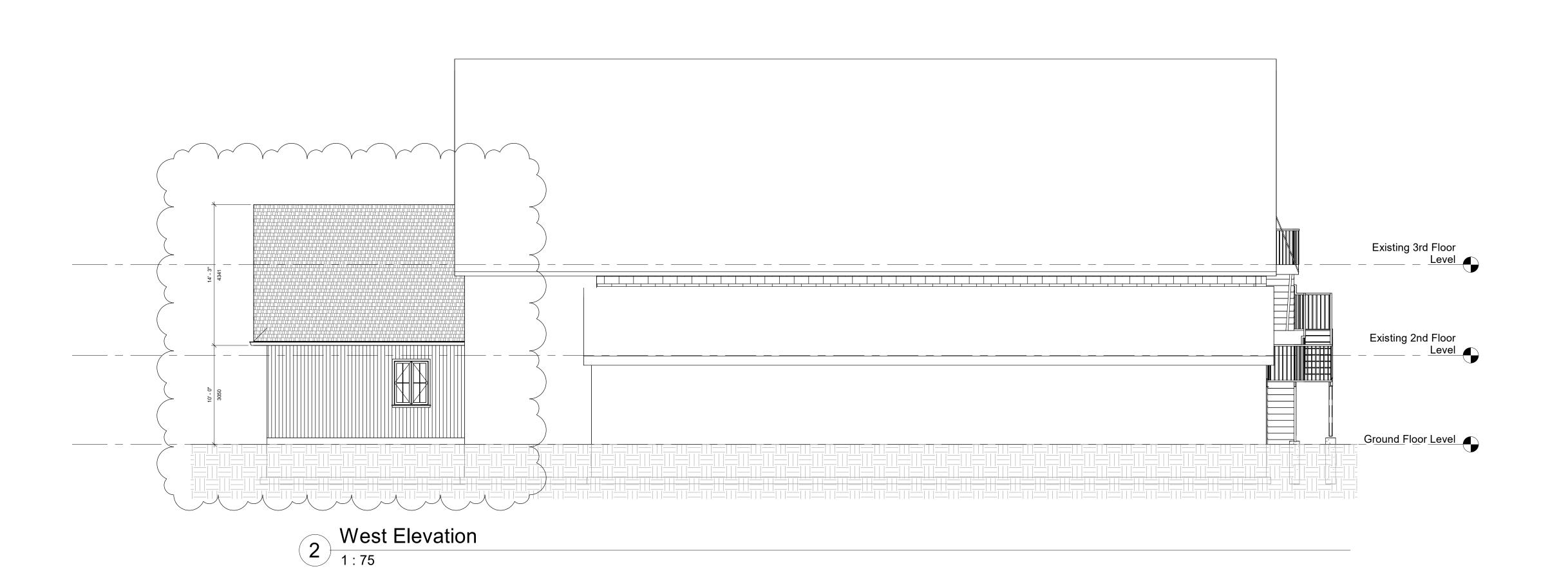
3rd Floor Level

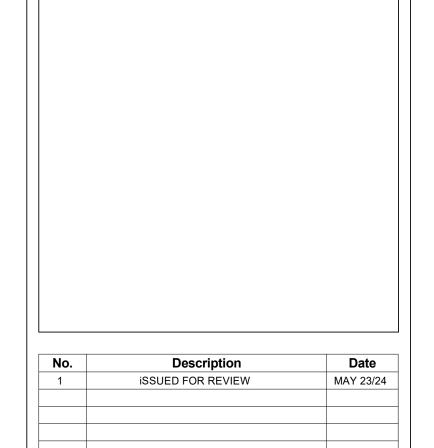
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14 VICTORIA STREET UXBRIDGE, ON

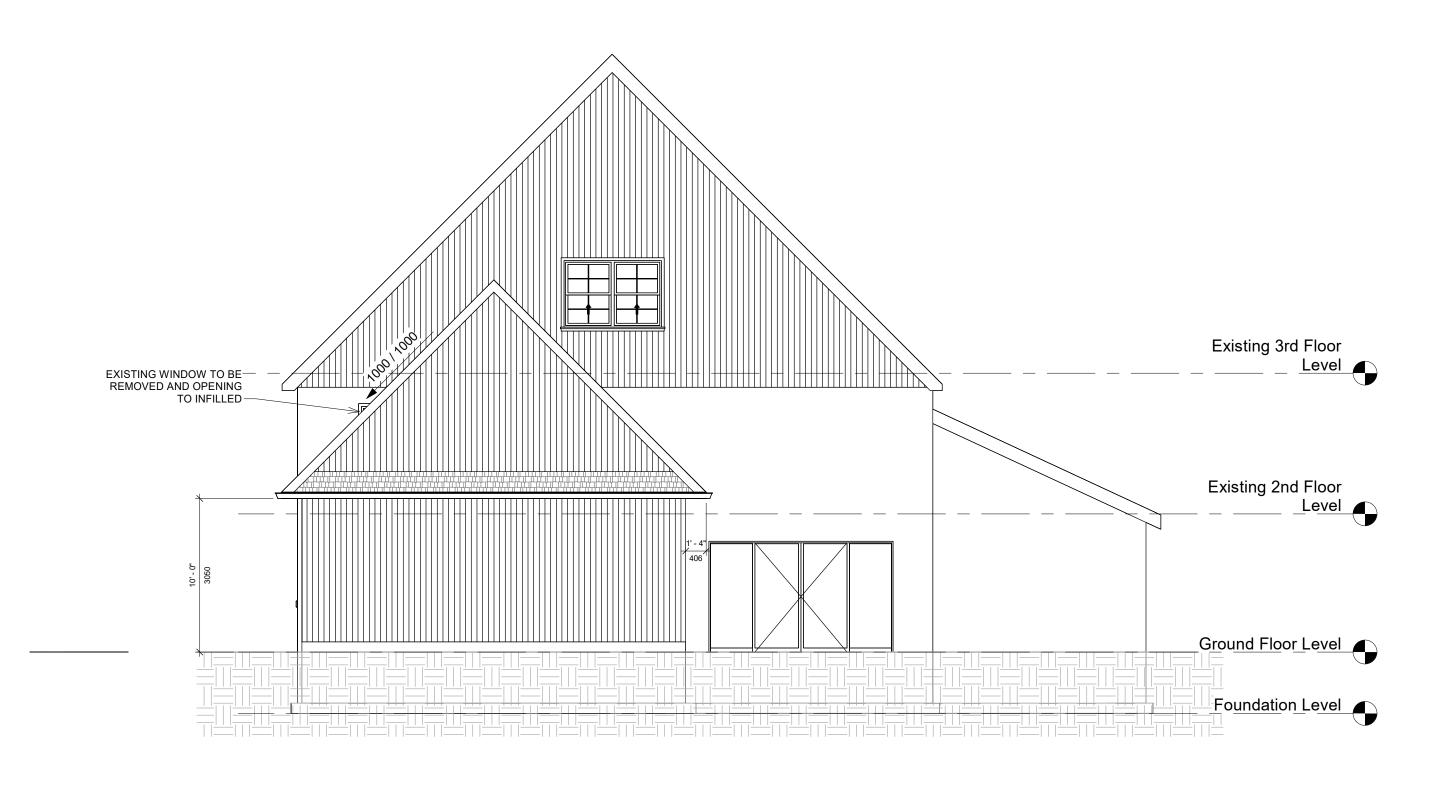
Elevations

Project number	24-031
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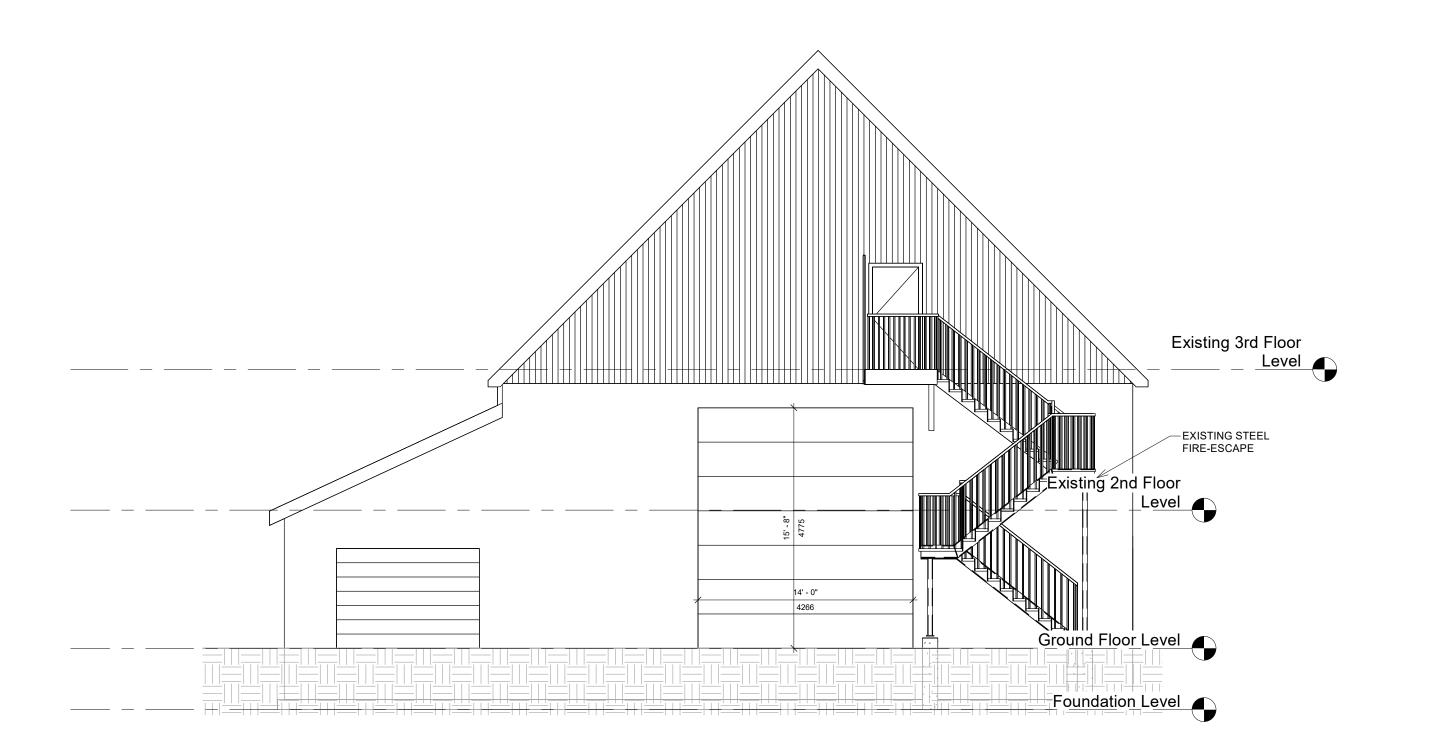
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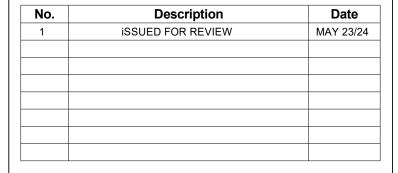
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North Elevation
1:75

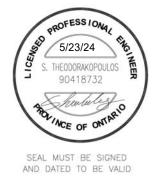


2 South Elevation
1:75



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PROPOSED ADDITION

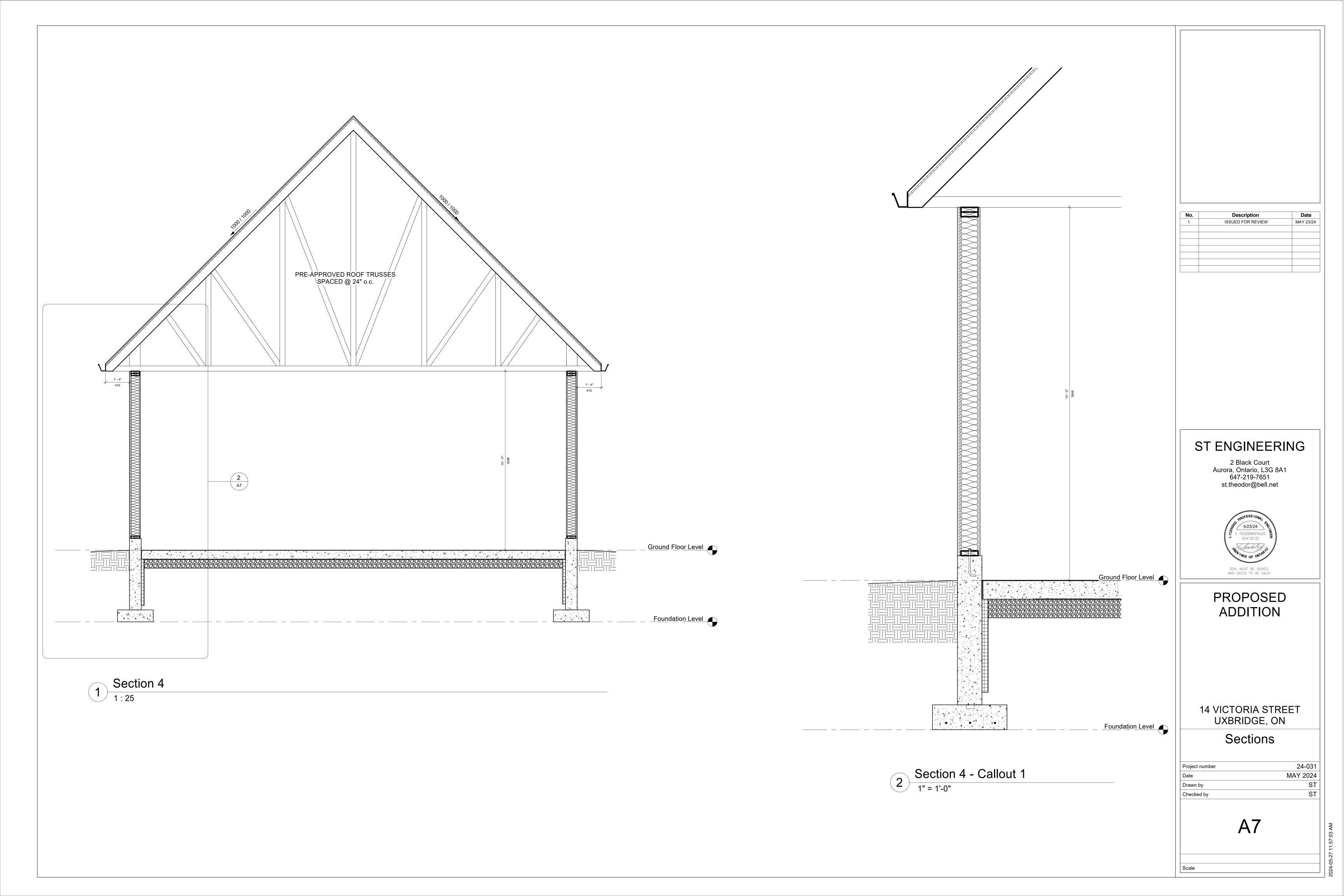
14 VICTORIA STREET UXBRIDGE, ON

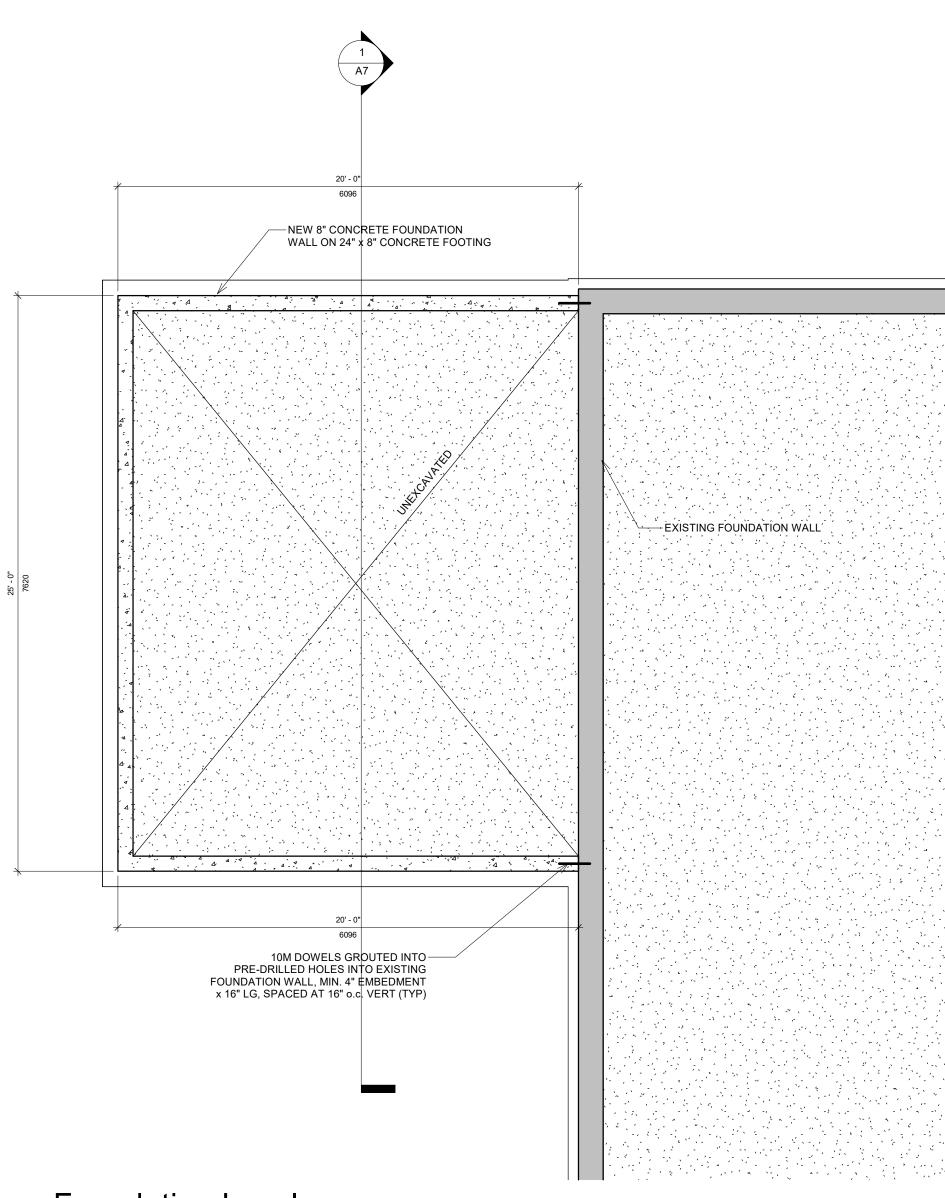
Elevations

Project number	24-031
Date	MAY 2024
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A6

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Foundation Level
1:50

No. Description Date

1 ISSUED FOR REVIEW MAY 23/24

ST ENGINEERING

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PROPOSED ADDITION

14 VICTORIA STREET UXBRIDGE, ON

Foundation Plan

Project number	24-031
Date	MAY 2024
Drawn by	ST
Checked by	ST

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Scale

DESIGN NOTES

- 1. THE STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH THE REQUIREMENTS OF THE 2012 ONTARIO BUILDING CODE.
- 2. ALL REINFORCED CONCRETE ELEMENTS HAVE BEEN DESIGNED IN ACCORDANCE WITH A23.3, DESIGN OF CONCRETE STRUCTURES.
- 3. ALL STRUCTURAL STEEL ELEMENTS HAVE BEEN DESIGNED IN ACCORDANCE WITH CAN/CSA-S16.1, LIMIT STATES DESIGN OF STEEL STRUCTURES.
 - A) THE STRUCTURAL STEEL DESIGN IS BASED ON "SIMPLE" CONSTRUCTION. AREAS WHERE "CONTINUOUS" CONSTRUCTION HAS BEEN USED HAVE BEEN IDENTIFIED ON THE PLANS.
 - B) THE STEEL STRUCTURE HAS BEEN DESIGNED TO PROVIDE DUCTILE RESPONSE UNDER SEISMIC LOADING. THE LATERAL LOAD RESISTED BY DUCTILE BRACED FRAMES.
 - ALL CONNECTIONS ARE ASSUMED TO BE BEARING TYPE CONNECTIONS, UNLESS OTHERWISE NOTED.
 THE BOLTS SHALL BE BROUGHT TO A SNUG-TIGHT CONDITION AS DEFINED IN CAN/CSA-S16.1.
- 4. ALL STRUCTURAL ALUMINUM ELEMENTS HAVE BEEN DESIGNED IN ACCORDANCE WITH CAN3-S157. STRENGTH DESIGN IN ALUMINUM.
- 5. ALL COLD FORMED STEEL STRUCTURAL MEMBERS HAVE BEEN DESIGNED IN ACCORDANCE WITH S136 COLD FORMED STEEL STRUCTURAL MEMBERS.

LIVE LOADS ON ROOFS

- 1. THE ROOF AREAS HAVE BEEN DESIGNED TO RESIST SNOW, RAIN AND WIND LOADS IN ACCORDANCE WITH THE 2012 ONTARIO BUILDING CODE / 2015 NATIONAL BUILDING CODE OF CANADA, WHICHEVER PRODUCES THE MORE UNFAVOURABLE EFFECT. THE DESIGN PARAMETERS FOR THESE LOADS ARE AS NOTED BELOW:
- 2. SNOW LOAD
 - A) THE GROUND SNOW LOAD OF 2.4 kPa AND ASSOCIATED RAIN LOAD OF 0.4 kPa, MODIFIED AS REQUIRED OR PERMITTED BY CODE, HAVE BEEN CONSIDERED IN THE DESIGN OF THE ROOF AREAS.
 - B) ADDITIONAL SNOW ACCUMULATION ADJACENT TO HIGHER WALLS, ROOF AND MECHANICAL UNITS HAS BEEN ACCOUNTED FOR.

RAIN LOAD

- A) THE DESIGN OF THE ROOF STRUCTURE IS BASED ON THE ASSUMPTION THAT THE FLOW CONTROL ROOF DRAINS SATISFY ALL REQUIREMENTS OF 2015 NATIONAL PLUMBING CODE OF CANADA.
- B) THE TOTAL LOAD ASSOCIATED WITH THE 24 HOUR RAINFALL, IN ACCORDANCE WITH THE ONTARIO BUILDING CODE IS 97mm OF WATER OVER THE ENTIRE ROOF AREA.
- C) THE ACTUAL DISTRIBUTION OF THIS LOAD HAS BEEN ADJUSTED TO ACCOUNT FOR THE ACTUAL ROOF SLOPES AND PROFILE.

4. WIND UPLIFT ON ROOFS

A) ROOF ELEMENTS I.E. TRUSSES, JOISTS, STEEL DECK, ETC.. AND THEIR CONNECTIONS TO THE STRUCTURE ARE TO BE DESIGNED FOR UPWARD SUCTION DUE TO WIND. THE UNFACTORED NET UPWARD DESIGN PRESSURES ARE SHOWN ON THE KEY PLAN OF ROOF FRAMING PLAN.

SERVICEABILITY CRITERIA

SIMPLE SPAN MEMBERS OF FLOORS AND

FINISHED SUSCEPTIBLE TO CRACKING

ROOFS SUPPORTING CONSTRUCTION AND

1. TYPICAL HORIZONTAL ELEMENTS (NOT SUPPORTING CLADDING) HAVE BEEN DESIGNED SO THAT THE THEORETICAL DEFLECTIONS WILL NOT EXCEED THE FOLLOWING VALUES:

TYPE OF MEMBER	DEFLECTION TO BE CONSIDERED	DEFLECTION LIMIT
REINFORCED CONCRETE MEMBERS		
FLAT ROOFS NOT SUPPORTING NON- STRUCTURAL ELEMENTS LIKELY TO BE DAMAGED BY LARGE DEFLECTIONS	IMMEDIATE DEFLECTION DUE TO SPECIFIED LIVE LOAD, L	L/180
FLOORS NOT SUPPORTING NON- STRUCTURAL ELEMENTS LIKELY TO BE DAMAGED BY LARGE DEFLECTIONS	IMMEDIATE DEFLECTIONS DUE TO SPECIFIED LIVE LOAD, L	L/360
ROOF OR FLOOR CONSTRUCTION SUPPORTING NONSTRUCTURAL ELEMENTS LIKELY TO BE DAMAGED BY LARGE DEFLECTIONS	THAT PART OF THE TOTAL DEFLECTION OCCURING AFTER ATTACHEMENT OF NONSTRUCTURAL ELEMENTS (SUM OF THE LONG -TERM DEFLECTION DUE TO ALL SUSTAINED LOADS AND THE IMMEDIATE DEFLECTION DUE TO ANY ADDITIONAL LIVE L	L/480 .OAD
ROOF OR FLOOR CONTRUCTION SUPPORTING NONSTRUCTURAL ELEMENTS NOT LIKELY TO BE DAMAGED BY LARGE DEFLECTIONS	IMMEDIATE DEFLECTION DUE TO SPECIFIED LIVE LOAD, L AND TOTAL DEAD LOAD	L/240
STRUCTURAL STEEL MEMBERS		
SIMPLE SPAN MEMBERS OF FLOORS AND ROOFS SUPPORTING CONSTRUCTION AND FINISHES NOT SUSCEPTIBLE TO CRACKING	LIVE LOAD	L/300

2. PERIMETER OR SPANDREL ELEMENTS (SUPPORTING CLADDING) AND ELEMENTS SUPPORTING MASONRY WALLS, HAVE BEEN DESIGNED FOR AN ALLOWABLE DEFLECTION OF ONE HALF THE VALUES NOTED ABOVE.

LIVE LOAD

- THE STRUCTURE HAS BEEN DESIGNED ASSUMING THAT THE INSTALLATION OF NONSTRUCTURAL ELEMENTS SUCH AS CLADDING, MECHANICAL AND ELECTRICAL SERVICES AND THE LIKE, WILL NOT COMMENCE UNTIL AT LEAST ONE MONTH AFTER THE REINFORCED CONCRETE SLAB SUPPORTING THE NONSTRUCTURAL ELEMENTS HAS BEEN POURED.
- THE STRUCTURE HAS BEEN DESIGNED TO LIMIT THE MAXIMUM INTERSTOREY DRIFT UNDER 1/10 AVERAGE HOURLY WIND PRESSURE TO H/500, WHERE H IS THE FLOOR TO FLOOR HEIGHT BETWEEN TWO ADJACENT FLOORS. UNDER SEISMIT LOADS, THE INTERSTOREY DRIFT HAS BEEN LIMITED TO 0.02 Hs, WHERE Hs IS THE HEIGHT OF THE STOREY.
- 5. NON STRUCTURAL ELEMENTS SUCH AS CLADDING, MECHANICAL, AND ELECTRICAL SERVICES AND SUPPORTS, AND THE LIKE, MUST BE DESIGNED AND DETAILED TO ACCOMMODATE THE ANTICIPATED MOVEMENTS NOTED ABOVE.

CONCRETE: TABLE 1

		MEMBER/LOCATION	MIN. COMPRESSIVE STRENGTH AT 28 DAYS (MPa)	SLUMP (mm)	EXPOSURE CLASS	MAX W/C RATIO	AIR CONTENT (%)
SALS	FOOTINGS	FOUNDATION WALL FTGS	35	80 ± 30	C - 1	0.40	5 - 8
CHEMIC	FOOT	COLUMN FOOTINGS & CAPS	35	80 ± 30	C - 1	0.40	5 - 8
EXPOSED TO DE-ICING CHEMICALS THOUT FREEZING AND THAWING	WALLS	FOUNDATION WALLS	35	80 ± 30	C - 1	0.40	5 - 8
POSEI OUT FF	MS	STRUCTURAL BEAMS/SLABS	35	80 ± 30	C - 1	0.40	5 - 8
CONCRETE EXPOS WITH/WITHOUT	AND E	SLAB ON GRADE (NOT EXPOSED TO FREEZING AND THAWING)	35	80 ± 30	C - 1	0.40	5 - 8
CONC	SLABS	SLAB ON GRADE (EXPOSED TO FREEZING AND THAWING)	35	80 ± 30	C - 1	0.40	5 - 8
IICALS	INGS	FOUNDATION WALL FTGS	35	80 ± 30	C - 1	0.40	5 - 8
THAWING U/N	FOOTINGS	COLUMN FOOTINGS & CAPS	25	80 ± 30			
DE-ICIN	SONS	SHAFT	25	150 ± 30			
AND AND	S	CAP	25	80 ± 30			
IOT EXPOSED FREEZING AN	WALLS	FOUNDATION WALLS	30	80 ± 30			
CONCRETE NOT OR FR	BEAMS	STRUCTURAL BEAMS/SLABS	35	80 ± 30			
CONC	SLABS AND	SLAB ON GRADE	25	40 ± 20			
	(0)						

GENERAL NOTES

A. GENERAL

- 1. WHERE DOCUMENTS ARE REFERENCED IN THE GENERAL AND DESIGN NOTES, THEY SHALL BE THE LATEST EDITIONS, UNLESS OTHERWISE NOTED OR SHOWN.
- 2. READ STRUCTURAL DRAWINGS IN CONJUNCTION WITH SPECIFICATIONS AND ALL OTHER CONTRACT
- 3. BEFORE PROCEEDING WITH WORK, CHECK ALL THE DIMENSIONS SHOWN ON THE STRUCTURAL DRAWINGS AGAINST THE ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS AND REPORT DISCREPANCIES TO THE CONSULTANT. REFER TO THE ARCHITECTURAL AND OTHER DRAWINGS FOR LOCATIONS AND SIZES OF OPENINGS AND SLEEVES NOT SHOWN ON THE STRUCTURAL DRAWINGS. HOWEVER, OBTAIN THE ENGINEERS PRIOR APPROVAL BEFORE INSTALLING OPENINGS, SLEEVES, ETC, WHICH ARE NOT SHOWN ON STRUCTURAL DRAWINGS.
- 4. SEE ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS FOR LOCATIONS AND SIZED OF PITS, BASES, HOUSEKEEPING PADS, SUMPS, TRENCHES, DEPRESSIONS, GROOVES, CURBS, CHAMFERS AND SLOPES NOT SHOWN ON STRUCTURAL DRAWINGS.
- 5. HORIZONTAL AND VERTICAL DESIGN LOADINGS ARE NOTED. THEY SHALL NOT BE EXCEEDED DURING CONSTRUCTION.
- 6. TYPICAL STRUCTURAL DETAILS SHALL GOVERN THE WORK. IF DETAILS DIFFER ON OTHER DRAWINGS, THE MOST STRINGENT SHALL GOVERN.

B. MATERIALS

L/360

- CONCRETE: CONFORM TO THE REQUIREMENTS OF CAN/CSA-A23.1, AND THE FOLLOWING CONTAINED IN TABLE 1.
- 2. REINFORCEMENT: CONFORM TO CSA G30 SERIES, fy = 400 MPa FOR ALL CONCRETE AND MASONRY REINFORCEMENT EXCEPT THAT fy = 386 MPa FOR WELDED WIRE FABRIC. PROVIDE WELDED WIRE FABRIC IN FLAT SHEETS ONLY. ALL REINFORCEMENT IS TO BE 'BLACK' EXCEPT WHERE THE SUFFIX 'C' IS USED TO DESIGNATE EPOXY COATED REINFORCEMENT.
- 3. STRUCTURAL STEEL:
 - STRUCTURAL WIDE FLANGE AND WELDED WIDE FLANGE SHAPES (W, WWF) TO CONFORM TO CAN/CSA-G40.20/G40.21 GRADE 350W.
 - ANGLES, PLATED AND CHANNELS (L, C) TO CONFORM TO CAN/CSA-G40.20/G40.21 GRADE 300W.
 - HOLLOW STRUCTURAL SECTIONS (HSS) TO CONFORM TO CAN/CSA-G40.20/G40.21 GRADE 300W.
 - ANCHOR BOLTS: CONFORM TO ASTM A307 OR 300W THREADED ROD CONFORMING TO CSA G40.21-M, UNLESS
 - OTHERWISE NOTED.
- STRUCTURAL BOLTS, NUTS AND WASHERS: CONFORM TO ASTM A325M.
- STRUCTURAL ALUMINUM FRAMING: CONFORM TO CSA HA SERIES.
- 7. CONCRETE MASONRY UNITS: CONFORM TO CAN3-A165 SERIES, 15 MPa MINIMUM COMPRESSIVE STRENGTH BASED ON NET AREA.
- MORTAR: CONFORM TO CSA A179, TYPE 'S' FOR LOAD BEARING WALLS.
- 9. MASONRY GROUT: CONFORM TO CSA A179, 20 MPa MINIMUM COMPRESSIVE STRENGHT AT 28 DAYS, 250mm SLUMP, MAXIMUM AGGREGATE SIZE 10mm.
- 10. NON-SHRINK GROUT: 35 MPa MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS.
- 11. FOUNDATION INSULATION: EXTRUDED POLYSTYRENE WITH A MINIMUM COMPRESSIVE STRENGTH OF 0.24 MPa UNLESS OTHERWISE NOTED.

C. FOUNDATIONS

PRESSURE OF 400 kPa.

- 1. A COPY OF THE SOIL INVESTIGATIONS REPORT BY SOIL ENGINEERS LTD. DATED SEPTEMBER 2020, IS AVAILABLE FROM THE ENGINEER. READ THIS REPORT, VISIT THE SITE AND THOROUGHLY FAMILIARIZE YOURSELF WITH ALL SURFACE AND SUBSURFACE CONDITIONS. THIS INFORMATION IS GIVEN SOLELY AS A GUIDE. NO RESPONSIBILITY IS ACCEPTED BY THE OWNER OR THE CONSULTANT FOR ITS CORRECTNESS, NOR SHALL ITS ACCURACY OR ANY OMISSIONS AFFECT THE PROVISION OF THIS CONTRACT.
- 2. FOUND ALL FOOTINGS ON NATURALLY CONSOLIDATED UNDISTURBED SOIL CAPABLE OF SAFELY SUSTAINING AN UNFACTORED BEARING PRESSURE OF 200 kPa. IF THESE CONDITIONS DO NOT PREVAIL AT THE ELEVATIONS SHOWN, ADVISE THE ENGINEER BEFORE PROCEEDING WITH THE WORK.
- 3. FOUND ALL CAISSONS ON SOUND UNDISTURBED SOIL CAPABLE OF SUSTAINING AN UNFACTORED BEARING
- 4. FOUND EXTERIOR FOOTINGS AND OTHER FOOTINGS SUSCEPTIBLE TO DAMAGE RESULTING FROM FROST ACTION A MINIMUM OF 1.2m BELOW FINISHED GRADE IF NOT NOTED TO BE FOUNDED LOWER.
- 5. PROVIDE TEMPORARY FROST PROTECTION, DURING CONSTRUCTION, FOR ALL FOOTINGS WHICH ARE NOT
- FOUNDED A MINIMUM OF 1.2m BELOW GRADE.
- THE EXISTING FOOTINGS, UNLESS NOTED OTHERWISE.

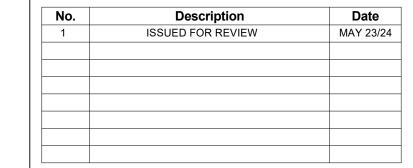
 7. INSULATION IS SHOWN WHERE REQUIRED FOR PROTECTION OF THE FOUNDATIONS FROM DAMAGE DUE TO

6. FOUND NEW FOOTINGS WHICH ARE LOCATED ADJACENT TO EXISTING FOOTINGS, AT THE SAME ELEVATION AS

- FROST ACTION ONLY. REFER TO ARCHITECTURAL DRAWINGS FOR FOUNDATION INSULATION NOT SHOWN ON THE STRUCTURAL DRAWINGS.
- THE LINE OF SLOPE BETWEEN ADJACENT FOOTINGS OR EXCAVATIONS OR ALONG STEPPED FOOTINGS SHALL NOT EXCEED A RISE OF 7 IN A RUN OF 10.
- DO NOT PLACE BACKFILL AGAINST WALLS RETAINING EARTH (OTHER THAN CANTILEVER WALLS) UNTIL THE WALLS AND THE FLOOR CONSTRUCTIONS AT TOP AND BOTTOM OF THE WALLS HAVE BEEN CAST AND ATTAINED 100% OF THEIR DESIGN STRENGTH.
- 10. WHERE THE SLAB ON GRADE IS USED TO TIE THE TOP OF A WALL RETAINING EARTH, THAT WALL SHALL BE ADEQUATELY SHORED UNTIL THE SLAB HAS BEEN CAST AND ATTAINED 100% OF ITS DESIGN STRENGTH.
- 11. CARRY OUT BACKFILLING AGAINST FOUNDATION WALLS WHERE THERE IS GRADE ON BOTH SIDES IN SUCH A MANNER THAT THE LEVEL OF BACKFILLING ON ONE SIDE OF THE WALL IS NEVER MORE THAN 500mm DIFFERENT FROM THE LEVEL ON THE OTHER SIDE OF THE WALL.
- 12. PROVIDE FOOTINGS FOR ALL MASONRY WALLS 240mm OR THICKER. 190mm MASONRY WALLS SHALL REST ON A THICKENED SLAB ON GRADE UNLESS OTHERWISE NOTED OR SHOWN.

D. SLAB ON GRADE

1. PLACE SLAB ON GRADE ON MATERIALS CAPABLE OF SUSTAINING 25 MPa WITHOUT SETTLEMENT RELATIVE TO THE BUILDING FOOTINGS.



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PROPOSED ADDITION

AND DATED TO BE VALID

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Notes

Project number	24-031
Date	MAY 2024
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